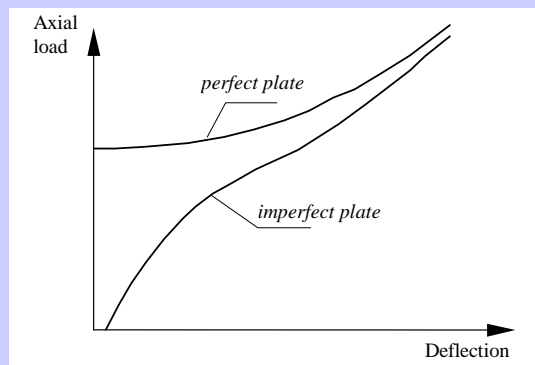
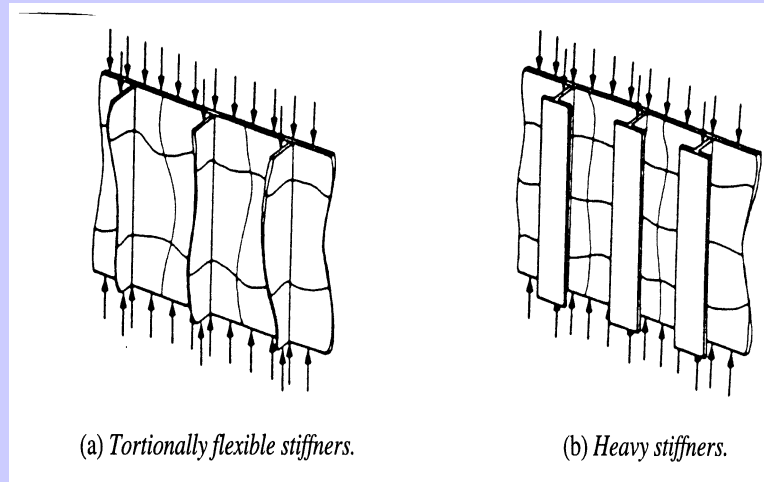


# Buckling of stiffened plates

## Important parameters

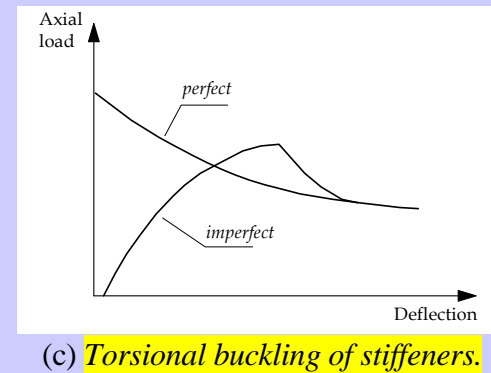
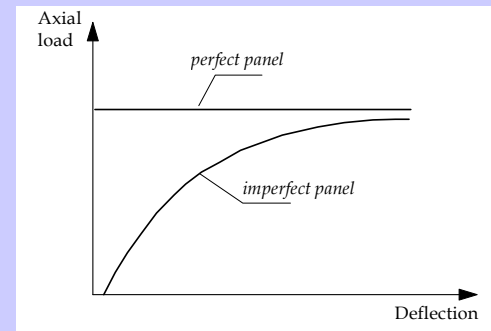
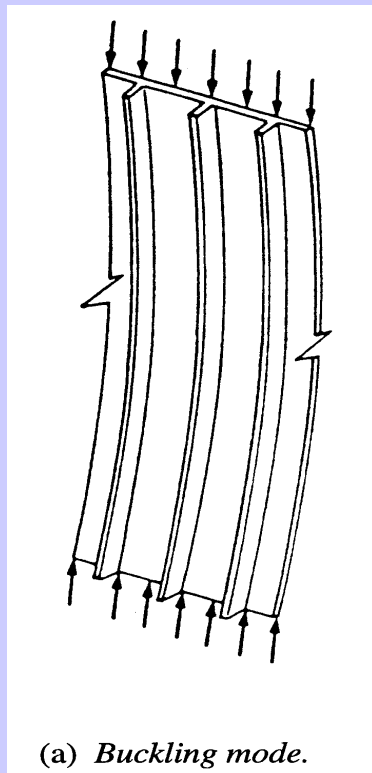
- length/width ratio of the panel
- stiffener geometry and spacing
- aspect ratio for plate between stiffener
- plate slenderness
- residual stresses
- initial distortions
- boundary conditions
- type of loading

# Buckling of short panel

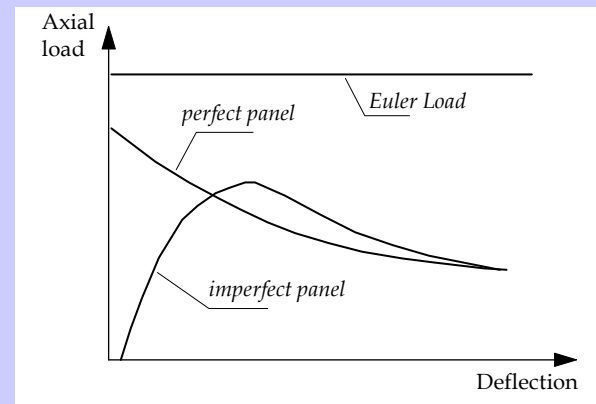
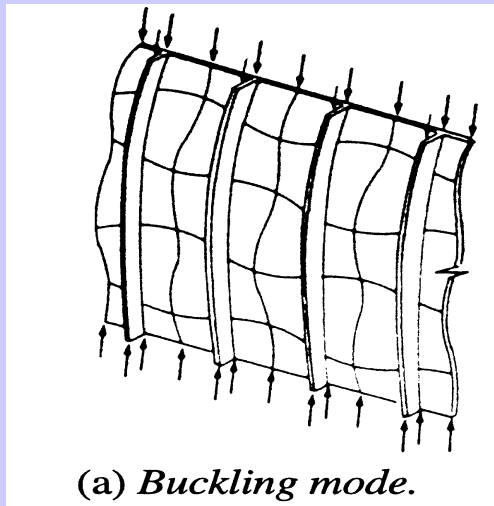


(c) Load-deflection behaviour of plate element.

# Buckling of long panel

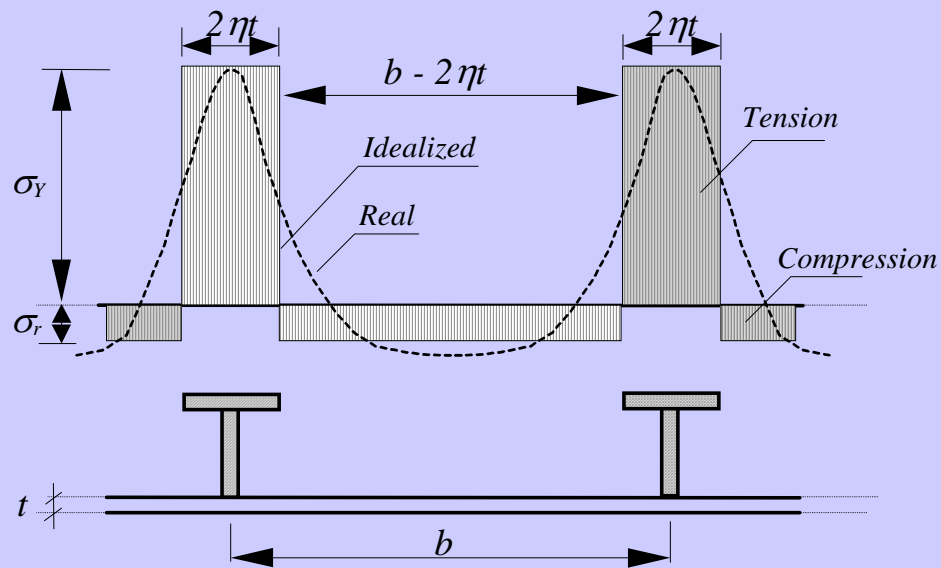


# Buckling of optimum panel



# Post-buckling capacity of plates

## Influence of in-plane restraint of long edges



Compressive stress  $\frac{\sigma_r}{\sigma_Y} = \frac{2\eta}{\frac{b}{t} - 2\eta}$

Reduction factor  $R_r = 1 - \frac{\sigma_r E_t}{\sigma_Y E} = 1 - \frac{2\eta}{\frac{b}{t} - 2\eta} \frac{2(\beta - 1)}{\beta}, 1 < \beta < 2.5$

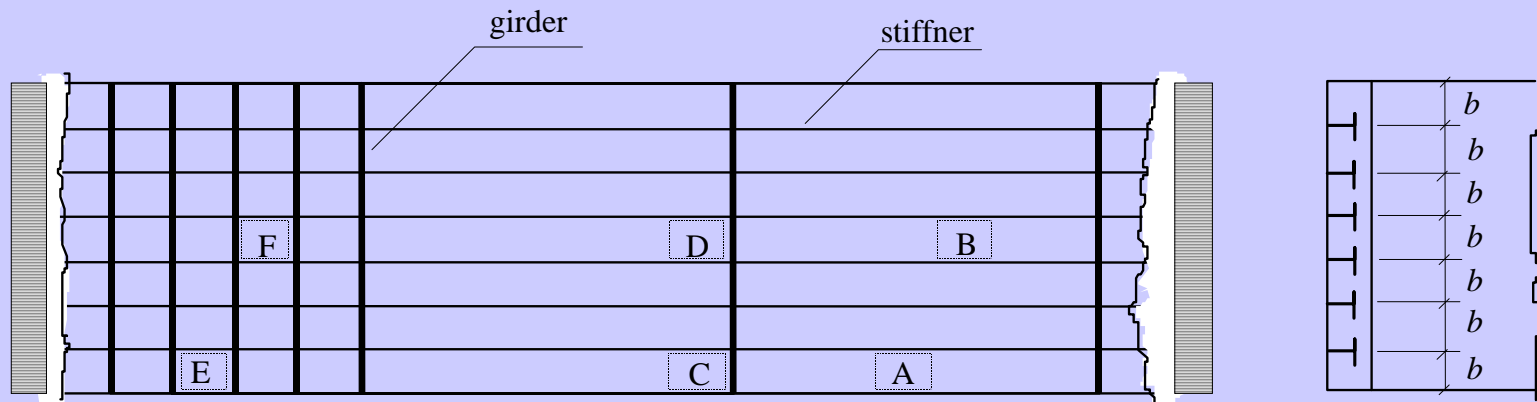
DnV Class. Note 30.1

$$\frac{b_e}{b} = \frac{\sigma_{xu}}{\sigma_Y} = \frac{1.8}{\beta} - \frac{0.8}{\beta^2} \quad \beta \geq 1$$

$$\frac{b_e}{b} = 1 \quad \beta \leq 1$$

# Post buckling capacity of plates

## Influence of boundary conditions



Boundary conditions depend on location of plate element

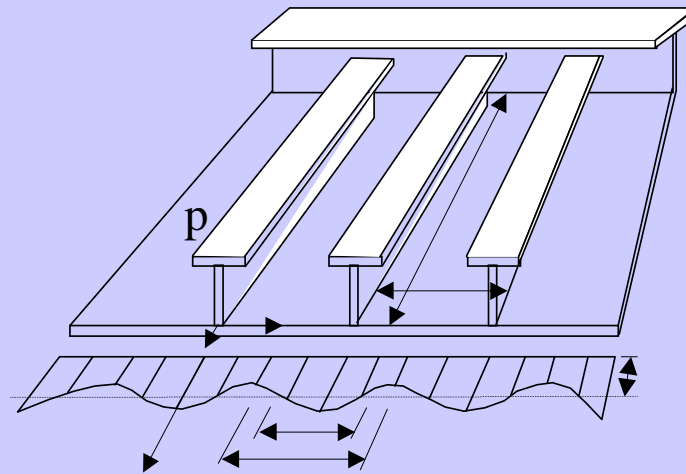
Classification example: Ship bottom

A: unrestrained

B: constrained

F: restrained

# Effective Width Concept



Effective width, long plate

$$\frac{b_e}{b} = \frac{\sigma_{xm}}{\sigma_y} = \begin{cases} \frac{2}{\beta} - \frac{1}{\beta^2} \beta \geq 1 \\ 1 \beta \leq 1 \end{cases}$$

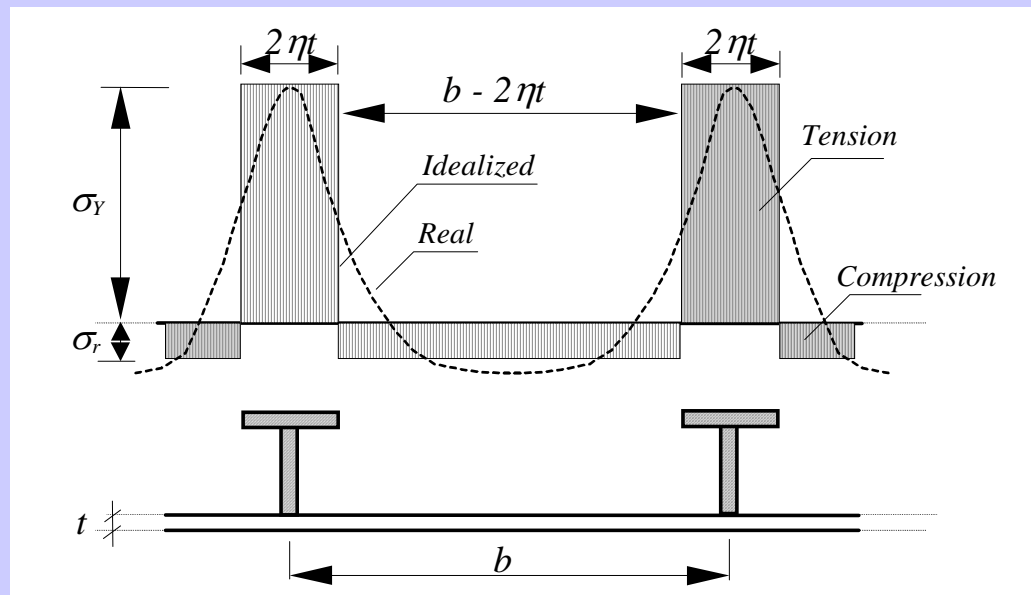
Effective width, short plate

$$\frac{a_e}{a} = \frac{\sigma_{ym}}{\sigma_Y} = \frac{0.9}{\beta^2} + \frac{1.9}{\alpha\beta} \left(1 - \frac{0.9}{\beta^2}\right), \alpha = \frac{a}{b}$$

Plate slenderness

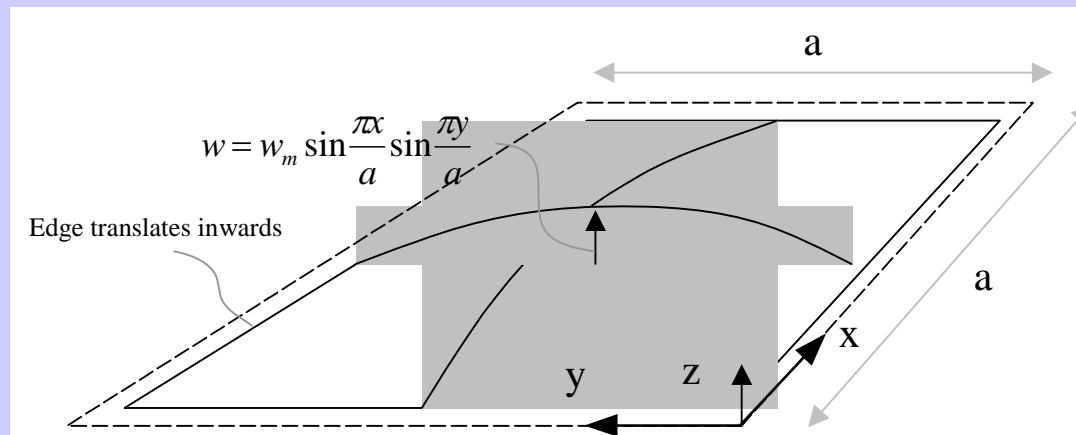
$$\beta = \frac{b}{t} \sqrt{\frac{\sigma_Y}{E}}$$

# Influence of residual stresses

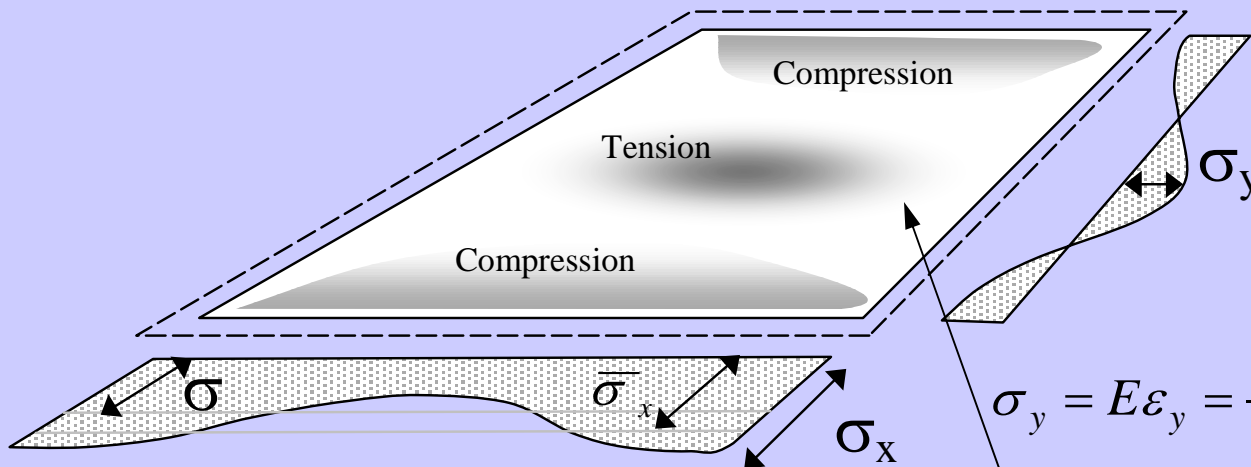




# Simple model for post-buckling capacity assumed displacement field



# Stress distribution along boundaries

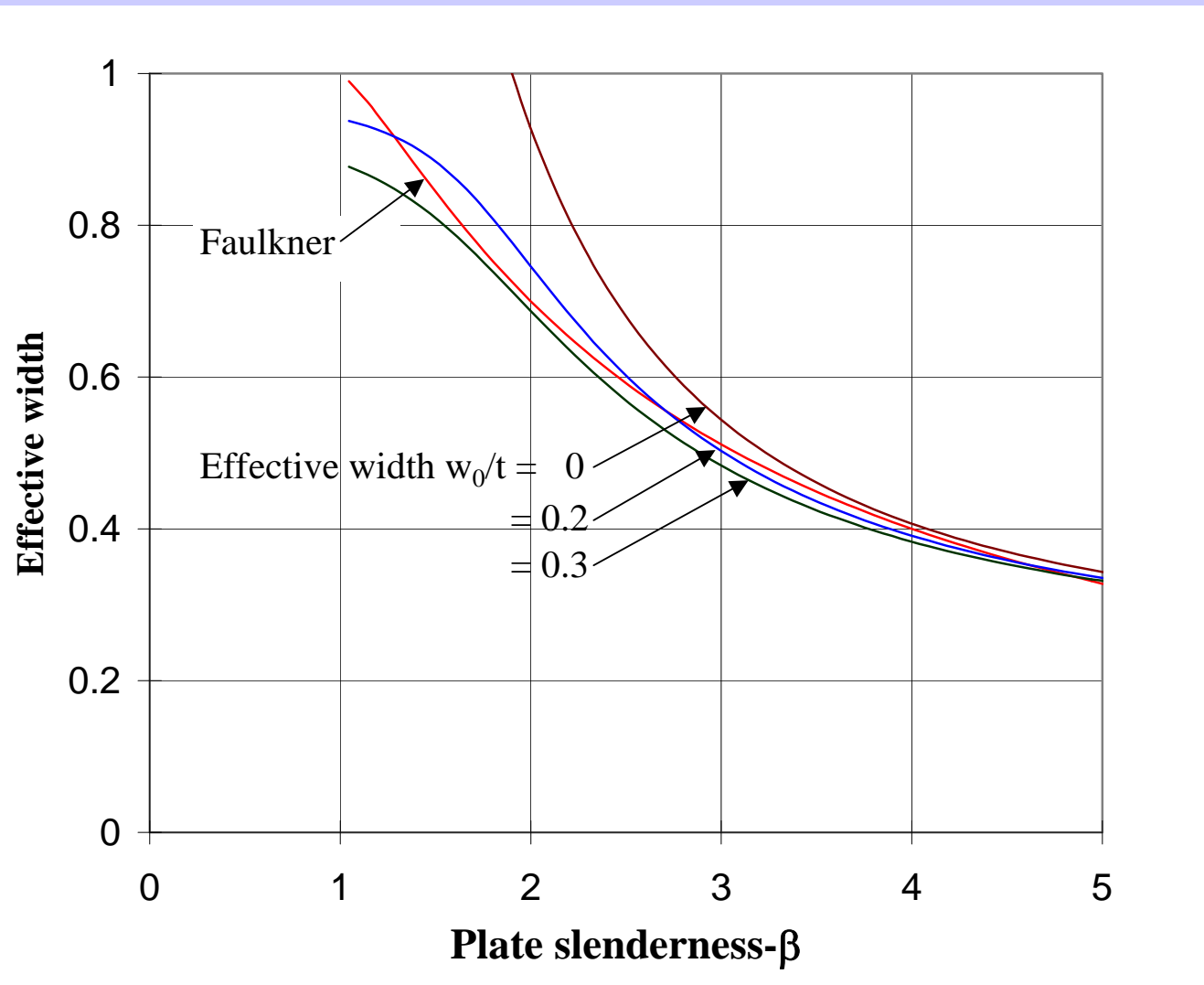


$$\sigma_y = E\varepsilon_y = -E \frac{\pi^2}{8} \left( \frac{w_m}{a} \right)^2 \cos \frac{2\pi x}{a}$$

$$\Delta\sigma_x(y) = -Et \frac{\pi^2}{16} \left( \frac{w_m}{a} \right)^2 \left\{ 1 + 2 \cos \frac{2\pi y}{a} \right\}$$

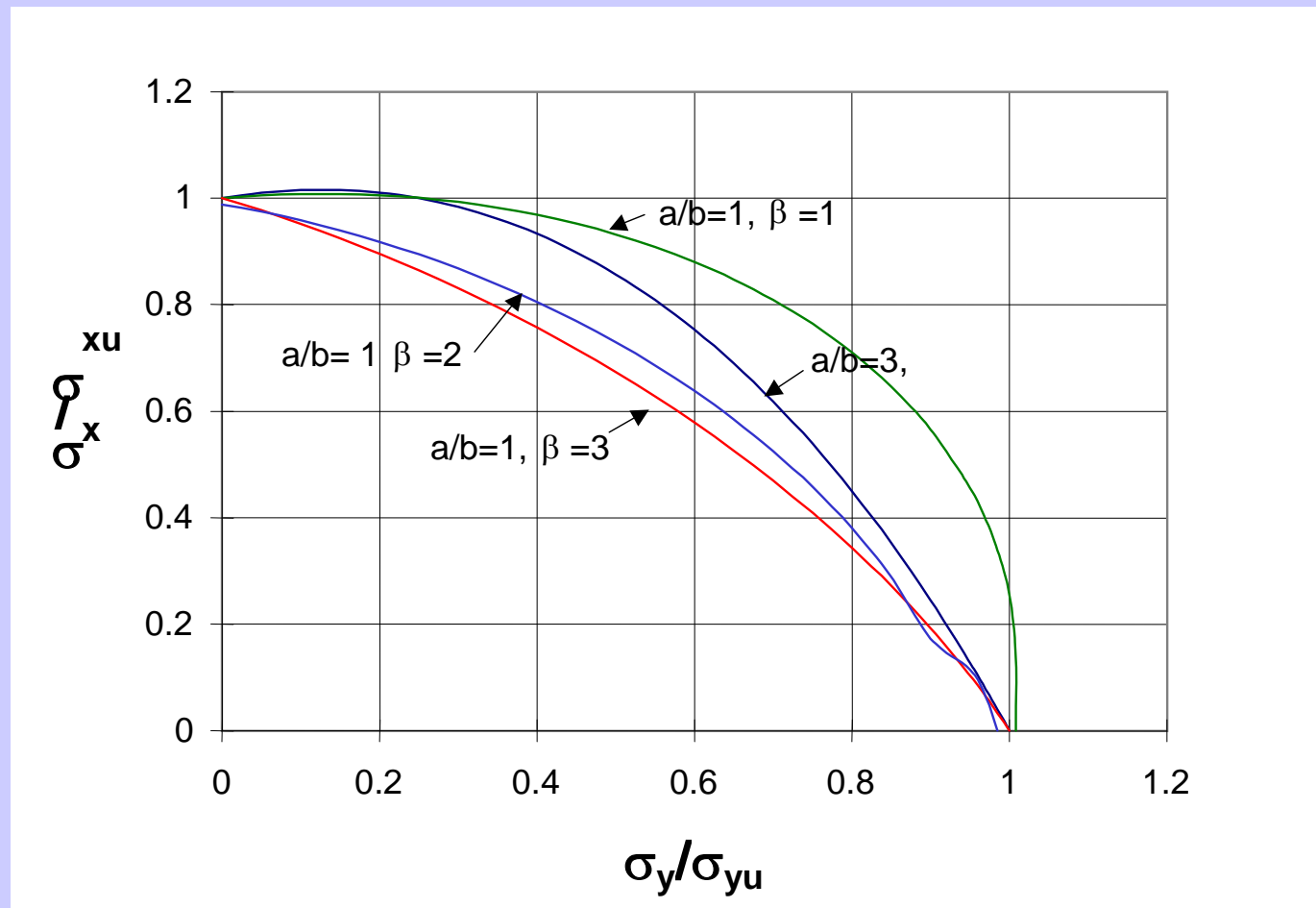
Failure criterion: Yielding

# Effective width -simple model



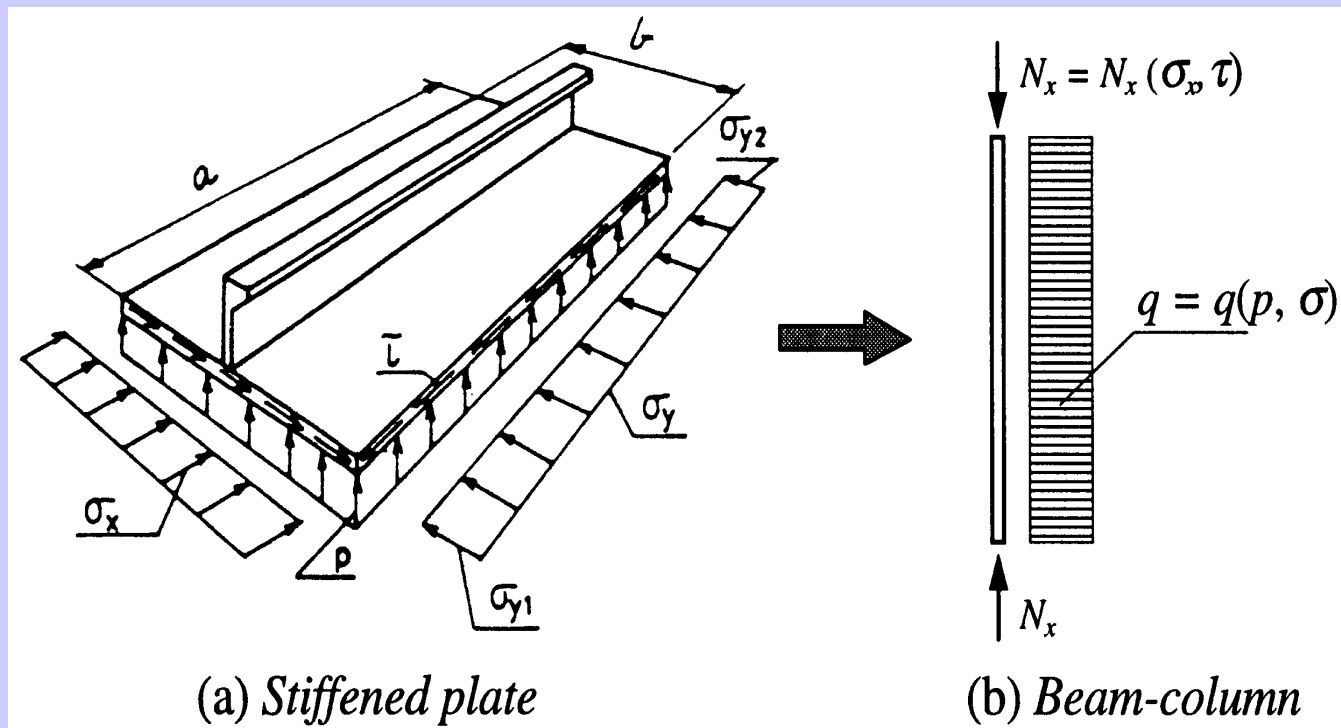
# Post-buckling capacity of plates

## Interaction curve for bi-axial compression (DnV)



# Stiffened plate

## Equivalent beam-column model



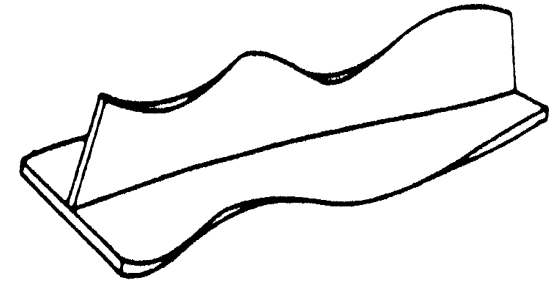
# Interframe collapse modes for stiffened plates



(a) *Plate induced.*



(b) *Stiffener induced.*



(c) *Tripping failure of stiffener.*